

Mechanics - Lecture 6 - 3/1/2022

[We'll start by discussing structural restrictions on the nonlinear elastic energy density W , from pp 5.8-5.11 of the Lecture 5 notes.]

A little more about the physical implications of a lack of rank-one convexity:

- Consider first a string in tension, with reference interval $0 < s < L$, $r(0) = 0$ and an applied force $(F, 0, 0)$ at $s = L$, with $F > 0$.



The assoc var'l problem is

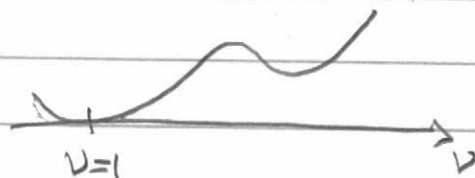
$$\min \int_0^L W(|\mathbf{r}'_s|) ds - F r_1(L).$$

Intuition suggests $\mathbf{r}'_2 = \mathbf{r}'_3 = 0$ and $\mathbf{r}'_1 = \nu > 1$. Since $r_1(L) = \int_0^L \mathbf{r}'_1$ this reduces the problem to

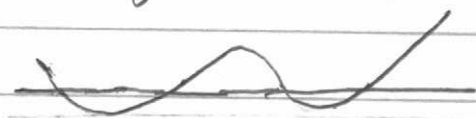
$$\min \int_0^L W(\nu(s)) - F \nu(s) ds$$

If the stress-strain law $\nu \rightarrow N(\nu) = W'(\nu)$ is not monotone for $\nu > 1$ then for some F , $\nu \rightarrow W(\nu) - F\nu$ will

have 2 local min of equal depth



$W(v)$



$W(v) - F_* v$

For $F < F_*$ the left min is preferred;
for $F > F_*$ the right min is preferred;
for $F = F_*$ either one is OK, and so
is any mixture. (Mixtures are
possible since a piecewise
constant v integrates to a
piecewise affine $\bar{v}(x)$.)

In HW1, this situation occurred
in problem 3(d) [the uniform ring
under pressure, if the stress-
strain law is non monotone].

In HW3, something similar is at
the heart of problem 1(d), (e), except
that there it's the isobaric stretch
 λ that plays the role of v .

- In 3D nonlinear elasticity we can
consider a similar setup with
energy

$$\int_{\Omega} W(Du) dx - \int_{\partial\Omega} \sum_i u_i G_i N_i ds$$

where G is a constant matrix (physically, this means we apply force $G \cdot N$ per unit reference area at $\partial\Omega$). Since this is equivalent to

$$\int_{\Omega} W(Du) - \sum_{ix} G_{ix} \frac{\partial u_i}{\partial x_x} \partial x$$

we are in a situation very much like the 1D bullet.

But: the higher-dimensional setting is different from 1D, since if u is const's then only special jumps of Du are permitted

$$Du = A \Big|_{\rightarrow \vec{n}} Du = B \quad \Rightarrow (B - A) \cdot \vec{\tau} = 0 \quad \text{whenever } \vec{\tau} \perp \vec{n}$$

$$\Rightarrow B - A = \vec{a} \otimes \vec{n} \quad \text{for some vector } \vec{a}$$

(strictly)
If W is rank-one convex then the phenomenon seen in the 1D setting (with a nonmonotone stress strain law) cannot occur.

Now we turn to linear elasticity. This entails two distinct linearizations:

1st geometrical linearization ("small strain theory") - we suppose

$$\chi_\delta(X) = X + \delta u(X)$$

with δ small. [Note that we work near the identity - not near an arbitrary rotn.] Here u is the "infinitesimal elastic displacement." Clearly

$$F = I + \delta \cdot Du$$

$$(F^T F)^{1/2} = I + \frac{\delta}{2} (Du + Du^T) + O(\delta^2)$$

so to leading order $(F^T F)^{1/2} - I$ is δ times

$$e(u) = \frac{1}{2} (Du + Du^T)$$

(the "linear elastic strain").

2nd physical linearization ("linear stress-strain law") - using hyperelasticity as a starting pt with $W(I) = 0$

and $W > 0$ except at rotations,
we have

$$W(F) = \delta^2 \langle \chi e(u), e(u) \rangle + \mathcal{O}(\delta^3)$$

where $\langle \chi e, e \rangle$ is the quadratic form assoc
with the Hessian of W . Evidently
(assuming some nondegeneracy)

$$\langle \chi e, e \rangle = \sum \chi_{ijkl} e_{ij} e_{kl}.$$

is a positive definite (symmetric)
quadratic form on symmetric matrices.

Dropping terms of higher order in δ
(and rescaling by δ^2) we expect a
var'l prin of the form $\delta E = 0$ where

$$E = \int_{\Omega} \frac{1}{2} \langle \chi e(u), e(u) \rangle dx + \left[\text{terms assoc} \right]_{\text{to loads}}$$

so equil eqn is

$$\operatorname{div}(\chi e(u)) + f = 0$$

and we recognize

$$\sigma = \lambda \epsilon(u)$$

as the stress tensor in the linearly elastic setting. We call λ its "Hooke's law".

Notes:

a) σ is symmetric

b) there is no distinction between Cauchy + Piola-Kirchhoff stress in linear elasticity. (Why not? Because we're working locally near $\chi(X) = X$. The reference + deformed volume + area elements agree at leading order in δ .)

Summary: in linear elasticity the basic unknown is the elastic displacement $u(x)$. Assoc lin strain is $\epsilon(u) = \frac{1}{2}(Du + Du^T)$. Stress is $\sigma = \lambda \epsilon(u)$. Egn of equil is

$$\operatorname{div} \sigma + f = 0, \quad \sigma = \lambda \epsilon(u)$$

(f = force/unit vol = "body load"), augmented by suitable bc such as

• $u = u_0$ at $\partial\Omega$ ("displacement bc")

or

or

$$\bullet \sigma \cdot n = g \text{ at } \partial\Omega \quad (\text{"traction bc"})$$

$$\bullet u \cdot n = 0, (\sigma \cdot n)_{\text{tan}} = 0 \text{ at } \partial\Omega \quad (\text{"lubricated bc"}).$$

We discussed isotropy in the nonlinear setting. The analogue for linear elasticity is

$$\begin{array}{l} \text{Hooke's law} \\ \alpha \text{ is isotropic} \end{array} \iff \alpha(R^T e R) = R^T \alpha(e) R \quad \begin{array}{l} \text{for any (symmetric)} \\ e \text{ and any (orientation} \\ \text{preserving) rotn } R \end{array}$$

(More generally: a rotn R is a symmetry of the material if $\alpha(R^T e R) = R^T \alpha(e) R$. For any symmetric e .)

Lemma: The general isotropic Hooke's law can be expressed as

$$\sigma_{ij} = 2\mu e_{ij} + \lambda (\text{tr } e) \delta_{ij}$$

where λ & μ are constants (Lamé moduli).

The proof is in every textbook. Rather than prove it here, let me suggest why it's

true:

Observe that Hooke's law is isotropic $\Leftrightarrow$
 $\langle \alpha e, e \rangle$ is a quadratic, isotropic fn of symmetric
 tensor, e (that is: it has the same value
 at e and at $R^T e R$.)

Fact of tensor algebra: a basis for such fn is
 obtained by starting from

$$e_{ij} e_{kl}$$

and "contracting indices in pairs."
 There are two ways to do this:

$$\bullet \sum_{i,k} e_{ii} e_{kk} = (\text{tr } e)^2$$

$$\bullet \sum_{i,j} e_{ij} e_{ij} = \text{tr}(e^2) = |e|^2$$

So isotropy $\Rightarrow$

$$\langle \alpha e, e \rangle = 2\mu |e|^2 + \lambda (\text{tr } e)^2$$

for some λ and μ .

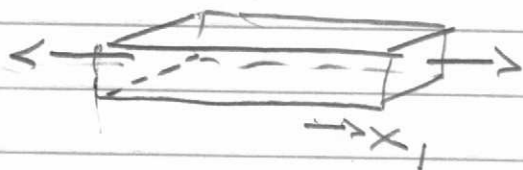
There are other useful reps of an
 isotropic Hooke's law, with clearer

physical interpretations:

- one uses Young's modulus E and Poisson's ratio ν

$$\sigma_{ij} = \frac{E}{1+\nu} \left(e_{ij} + \frac{\nu}{1-2\nu} (\text{tr } e) \delta_{ij} \right)$$

To see meaning of $E + \nu$, consider behavior under uniaxial tension



$\sigma_{11} = T$,
other components
of $\sigma = 0$

$$\Rightarrow e_{11} = \frac{1}{E} T$$

$$e_{22} = e_{33} = -\frac{\nu}{E} T$$

Most materials have $\nu > 0$, so uniaxial tension produces contraction in the orthog variables (and uniaxial compression produces expansion). Cork is different: it has $\nu \approx 0$, which is why it is useful for closing wine bottles.

- bulk and shear moduli describe

how α behaves on the two rotation-invariant subspaces

$\{\text{multiples of } \mathbf{I}\}$ and $\{\mathbf{e} : \text{tr } \mathbf{e} = 0\}$;

specifically:

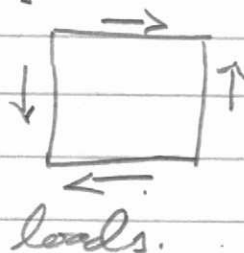
$$\sigma_{ij} = 3K \cdot \underbrace{\frac{1}{3}(\text{tr } \mathbf{e}) \delta_{ij}}_{\substack{\text{proj of} \\ \mathbf{e} \text{ to multiples} \\ \text{of } \mathbf{I}}} + 2\mu \underbrace{(\mathbf{e}_{ij} - \frac{1}{3}(\text{tr } \mathbf{e}) \delta_{ij})}_{\substack{\text{proj of } \mathbf{e} \\ \text{orthog to } \mathbf{I}}}.$$

Physical interpretation:

- bulk modulus controls how volume changes in response to hydrostatic pressure
- shear modulus describes response to "pure shear load"

eg $\sigma_{12} = T$, $\sigma_{ij} = 0$ otherwise

2D picture
in x_1, x_2 plane



$$\Rightarrow e_{12} = \frac{1}{2\mu} \sigma_{12} \quad (e_{ij} = 0 \text{ otherwise})$$

in 2D, this example gives $Du = \frac{1}{2u} \begin{pmatrix} 0 & 1 \\ 1 & 0 \end{pmatrix}$
(one diagonal is stretched, the other is shrunk)

[Caution: in 2D, projn of e to multiples of I is $\frac{1}{2}(\text{tr } e)I$.]

Of course these different expressions for α are related:

$$\lambda = \frac{E\nu}{(1+\nu)(1-2\nu)} \quad \mu = \frac{E}{2(1+\nu)}$$

$$\lambda = K - \frac{2}{3}\mu$$

Positivity of $\langle \alpha e, e \rangle$ requires

$\mu > 0$	$E > 0$	$\mu > 0$
$3\lambda + 2\mu > 0$	$-1 < \nu < 1/2$	$K > 0$
for Lamé moduli	for Young's modulus + Poisson's ratio	for bulk + shear moduli.

For analysis (and numerics), vari'l principles are very important. Simplest examples:

Dir bc φ

$$\min_{u=\varphi \text{ at } \partial\Omega} \int_{\Omega} \frac{1}{2} \langle \epsilon(u), \epsilon(u) \rangle - \langle u, f \rangle dx$$

Traction bc ($\sigma \cdot n = g$ at $\partial\Omega$)

$$\min_u \left\{ \int_{\Omega} \frac{1}{2} \langle \epsilon(u), \epsilon(u) \rangle - \langle u, f \rangle dx - \int_{\partial\Omega} \langle u, g \rangle ds \right\}$$

Both are convex - but degenerate since the quadratic term sees only the symmetric part of Du . So it is natural to ask:

Q1) if $\int_{\Omega} |\epsilon(u)|^2 < \infty$, is $u \in H^1(\Omega)$?

Q2) if $\epsilon(u) \equiv 0$, must u be an "infinitesimal rigid motion" ($u = M \cdot x + b$ with M a constant skew-symmetric matrix)?

Korn's inequality gives the answers (and more). We'll discuss it next week.